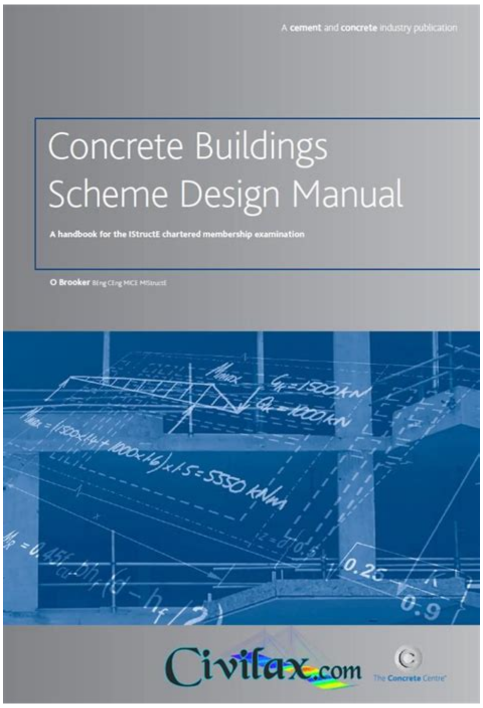

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A more detailed guide to the design of PT floors can be found in The Concrete Societys Technical Report 43 TR43 Posttensioned concrete floors Design handbook. For details, see the corrigendum published on page 37 of the October 2015 issue. Please accept all cookies to continue to improve our service. The information is presented in terms of the design strategy, anatomy of building design and structural systems. In reality, building design is a synthesis of architectural, structural, services, logistics and buildability issues. Steel frames are ideally suited for modern multistorey commercial buildings. The process should begin with a clear understanding of the client requirements and of local conditions or regulations. Despite the complexity, it is possible to identify a hierarchy of design decisions. Firstly, planning requirements are likely to define the overall building form, which will also include aspects such as natural lighting, ventilation and services. The principal design choices that need to be made in close consultation with the client are It is at this early interactive stage where the important decisions are made that influence the cost and value of the final project. Close involvement with the client is essential.For example, a maximum column spacing of 6 m is normally required for brickwork. The span of the beams across the building normally conforms to one of the following column grid arrangement This is shown in the figure below Natural lighting also plays a role in choice of the width of floor plate. In modern buildings, a long span solution provides a considerable enhancement in flexibility of layout. For airconditioned offices, a clear span of 15 m to 18 m is often used. An example of the column grid for a long span option in a building with a large atrium is shown in the figure below. The following general guidance influences the choice of structure. Wider plan forms are not generally suitable for open plan office space.

Ultra tall buildings are influenced strongly by the stabilising system and are not covered here. Sizes of lifts and their speed of movement also become important considerations for tall buildings. Depending on the Regulations for fire safety, the use of sprinklers may be required for buildings of more than eight storeys or approximately 30 m high.Positioning of service and access cores is influenced by The design requirements for atria are The following target floortofloor depths should be considered at the concept design stage For buildings up to eight storeys height, the steel structure may be designed to provide stability, but for taller buildings, concrete or braced steel cores are more efficient structurally. The following structural systems may be considered for stability. This is generally only possible where the beams are relatively deep 400 mm to 500 mm and where the column size is increased to resist the applied moments. Full depth end plate connections generally provide the necessary connection stiffness.Cross bracing is often simple flat steel plate, but angle and channel sections may also be used. A steel braced frame has the two key advantages The concrete core is generally constructed in advance of the steel framework. In this form of construction, the beams often span directly between the columns on the perimeter of the building and the concrete core. Special structural design considerations are required for A double beam may be required to minimise the structural depth at the corner of the cores. Such cores are installed with the rest of the steelwork package. An example of a braced steel core is shown in the figure above middle. This publication provides a model for the design of simple connections that transfer shear force due to permanent and variable loads and a noncoincident axial tie force resulting from an accidental load case.

It points out additional issues which must be considered where coincident shear forces and axial forces are to be dealt with. A sample design of a simple connection for a 610 serial size UB is presented, and the design of punching shear reinforcement for the wall is included. The guide discusses the responsibilities of the building structural engineer and the steelwork contractor and suggests where the responsibilities are best divided. It also considers the impact of deviations between the theoretical positions of the parts of the connection and their aserected positions.When the stability of the structure is provided by cores, or discreet vertical bracing, the beams are generally designed as simply supported. The generally accepted design model is that nominally pinned connections produce nominal moments in the column, calculated by assuming that the beam reaction is 100 mm from the face of the column. If the reactions on the opposite side of the column are equal, there is no net moment. Columns on the perimeter of the structure will have an applied moment, due to the connection being on one side only. Typical internal column sizes are given in the table below. It can be difficult and costly to provide connection into the minor axis of a very small column section.This has the key advantage of columnfree space, allowing future adaptability, and fewer foundations. The supporting beams may be below the floor, with the floor bearing on the top flange often known as downstand beams , or the beams may share the same zone with the floor construction, to reduce the overall depth of the zone. The available construction zone is often the determining factor when choosing a floor solution. Beams may be noncomposite, or composite. In composite construction shear connectors are welded to the top flange of the beam, transferring load to the concrete floor. Long span steel options generally provide for service integration for spans of over 12 m.

Cellular beams and composite trusses are more efficient for long span secondary beams, whereas fabricated beams are often used for long span primary beams. The approximate quantities are presented in the table below, and are expressed in terms of the total floor area of the building.The structural scheme has a key influence on programme and cost, and structural solutions which can be erected safely, quickly to allow early access for the following trades. In city centres, a solution involving fewer, albeit more heavily loaded foundations are often preferred, which lead to longer spans for the superstructure. Multistorey structures are generally erected using a tower crane, which may be supplemented by mobile cranes for specific heavy lifting operations. In city centre projects, tower cranes are often located in a lift shaft or atrium.See SCI P166. Other systems provide conditioned air from within a raised floor.Large open spaces can be created, which are efficient, easy to maintain and are adaptable as demand changes. Single storey buildings are a core market for steel in the UK.Therefore, many factors have to be addressed in their design. Although these building types are primarily functional, they are commonly designed with strong architectural involvement dictated by planning requirements and client branding. For example, the requirements for a distribution centre will be different to a manufacturing facility. A review of the importance of various design issues is presented in the table on the right for common building types.Various examples are presented below together with a brief description of the design concept. The figure shows a conceptual crosssection through each type of building, with notes on the structural concept, and typical forces and moments due to gravity loads. Bracing will be required in the roof and all elevations, to provide inplane and longitudinal stability. A portal frame may be single bay or multi bay.

The members are generally plain rolled sections, with the resistance of the rafter enhanced locally with a haunch. In many cases, the frame will have pinned bases. Stability in the longitudinal direction is provided by a combination of bracing in the roof, across one or both end bays, and vertical bracing in the elevations. If vertical bracing cannot be provided in the elevations due to industrial doors, for example stability is often provided by a rigid frame within the elevation.The trusses may take a variety of forms, with shallow or steep external roof slopes. A truss building may also be designed as rigid inplane, although it is more common to provide bracing to stabilise the frame.These may be used in portalised structures, but are often used with rigid bases, and with bracing to provide inplane stability. External or suspended support structures may be used, but are relatively uncommon.Their efficiency depends on the method of analysis, and the assumptions that are made regarding the restraint to the structural members, as shown in the table below. Brickwork is often used as a dado or dwarf wall below the level of the windows for impact resistance.A singlespan symmetrical portal frame as illustrated in the figure below is typically of the following proportions The eaves height is determined by the specified clear height between the top of the floor and the underside of the haunch. The length of the haunch means that the hogging bending moment at the sharp end of the haunch is approximately the same as the maximum sagging bending moment towards the apex, as shown in the figure below. Gable frames may be identical to the internal frames, even though they experience lighter loads. If future extension to the building is envisaged, portal frames are commonly used as the gable frames, to reduce the impact of the structural works. A typical gable frame formed from columns, short simplysupported rafters and vertical bracing, is shown in the figure below.

In the longitudinal direction, stability is provided by vertical bracing in the elevations. The vertical bracing may be at both ends of the building, or in one bay only. Each frame is connected to the vertical bracing by a hotrolled member at eaves level. A typical bracing arrangement is shown in the figure below. If diagonal bracing in the elevations cannot be accommodated, longitudinal stability can be provided by a rigid frame on the elevation. The purlins and side rails are considered adequate to restrain the flange that they are attached to, but unless special measures are taken, the purlins and side rails do not restrain the inside flange. Restraint to the inside flange is commonly provided by bracing from the purlins and side rails, as shown in the figure below. The bracing is usually formed of thin metal straps, designed to act in tension, or from angles designed in compression if bracing is only possible from one side. The arrangement of restraints to the inside flange is generally similar to that shown in the figure below and in all cases, the junction of the inside face of the column and the underside of the haunch must be restrained. In almost all cases a compression stiffener in the column as shown, at the bottom of the haunch will be required. Other stiffeners may be required to increase the bending resistance of the column flange, adjacent to the tension bolts, and to increase the shear resistance of the column web panel. The haunch is generally fabricated from a similar size beam to the rafter or larger, or fabricated from equivalent plate. Typically, the bolts may be M24 8.8 and the end plate 25 mm thick S355.The apex connection primarily serves to increase the depth of the member to make a satisfactory bolted connection. The apex haunch is usually fabricated from the same member as the rafter, or from equivalent plate. Typically, the bolts may be M24 8.8 and the end plate 25 mm thick S355. What should the structure be made from.

Where should columns or walls be located. Is it a framed building or does it have loadbearing walls. How thick do walls and floors need to be Particularly useful for engineers who have designed buildings out of steel and concrete, but have limited experience with timber, it will also benefit architects, students and lecturers who need comparative information on structural timber options. The second edition of this book contains additional coverage of robustness, moisture movement, typical moment connection capacities, long span beams, as well as several new drawings, graphs and sizing tables. James Norman, Programme Director of Civil Engineering at Bristol University has researched and written this book with assistance from TRADA members. Ataglance reference guide to structural timber options for use during the early stages of design. Includes comparisons of sawn timber, glulam, Ijoists, unilam, CLT, LVL and flitch beams to aid product selection. Highly illustrated with drawings, graphs and sizing tables, enabling designers. Timber Bridges. Vibration in timber floors. MULTIPACK Structural timber elements a prescheme design guide 2nd edition 5 c. Clerk of Works and Site Inspector Handbook 2018 edition. Practical Building Conservation Glass and Glazing. External solar shading with wood a guide for specifiers. BS EN 14915 2013 Solid wood panelling and cladding. Characteristics, evaluation. Basic Structures 3rd edition. BS 84142 2020 Fire performance of external cladding systems. Test method for no. Hybrid construction timberbased solutions to structural challenges. External timber cladding 3rd edition. Green oak in construction. BS 55342014 Slating and tiling for pitched roofs and vertical cladding. Code of. Plywood a material story. WIS 132 Upgrading timber joinery doors for fire resistance. Seals for timber floors A specification guide. Woodbased panel products and timber in fire. Timber decking 3rd edition.